

# FOUNDATION ENGINEERING

## DEEP FOUNDATIONS

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## TYPES OF DEEP FOUNDATIONS:

1- PILES

الخوازيق

2- CAISSONS

القيسونات

أساسات خلوية تنفذ بالتفويض والحفر

3- PIERS

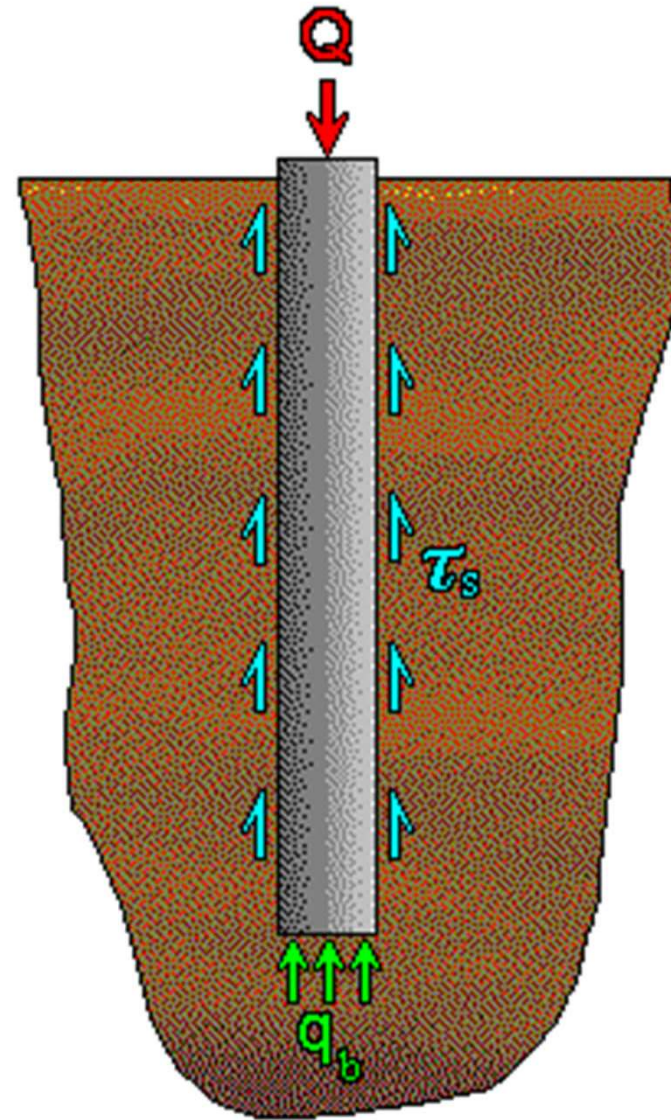
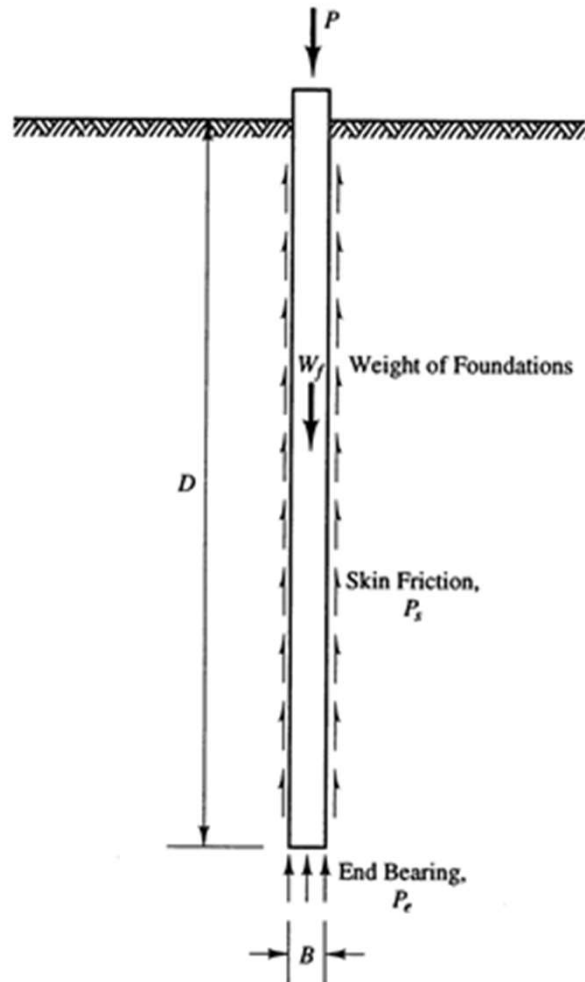
الدعائم

أساسات الكبارى وهى خوازيق ذات قطر كبير أو قواعد ذات حجم كبير يجفف داخلها

# **Pile are used in**

- 1- Upper soil is weak, compressible, or could not support the surface loads.**
- 2- The loads are tension, horizontal, or inclined.**
- 3- Problematic soils;**
  - Swelling soils giving tension on the pile.**
  - Collapsing soils, adding down-drag forces on the pile.**
- 4- Scour under bridge piers.**
- 5- Temporary or Permanent Excavation Side Support**

# Load Transfer





# Pile Foundations

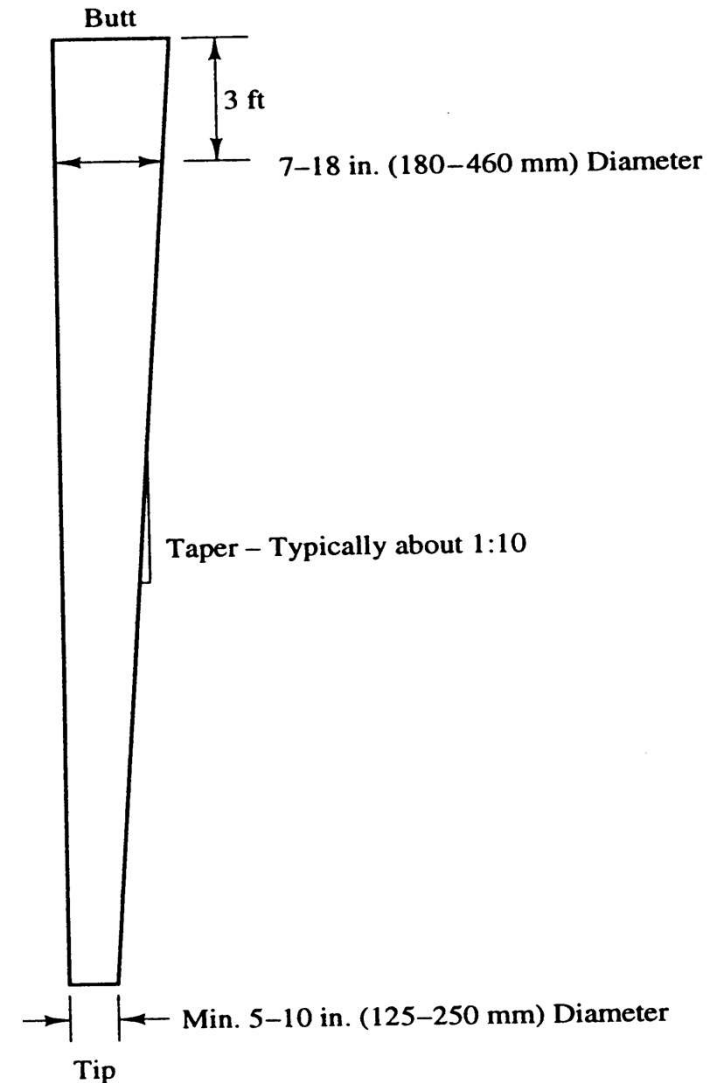
- Types of Piles
  - Materials
  - Typical Dimensions
- Installation Methods & Equipment
- Construction Procedures
- Typical Applications

# Types of Piles

- Timber Piles
- Steel Piles
- Concrete Piles
- Composite Piles

# Timber Piles

- Southern Pine
- Douglas Fir
- Tapered Shape
- Butt dia. 6-16 inches
- 20-60 ft length
- 20-100k capacity



# Timber Piles

- Usually treated with preservatives before installation (Creosote)
- Longer serviceable life if completely submerged in water; cyclic wetting & drying causes decay
- Susceptible to damage during driving
- Steel bands near butt, steel shoe at tip



# Steel Piles

- Easy to Splice
- Good choice when  $D > 60\text{ft}$
- Good choice in hard soils
- Expensive
- Noisier to drive
- Susceptible to Corrosion

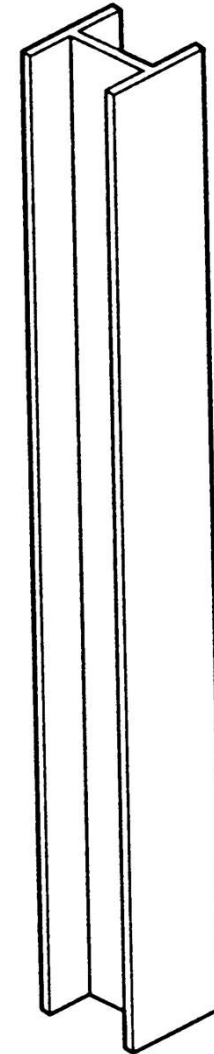
# Steel Piles

## ■ H-piles

- 50-150 ft length
- 80-400 kip working loads
- small displacement piles

## ■ Pipe Piles

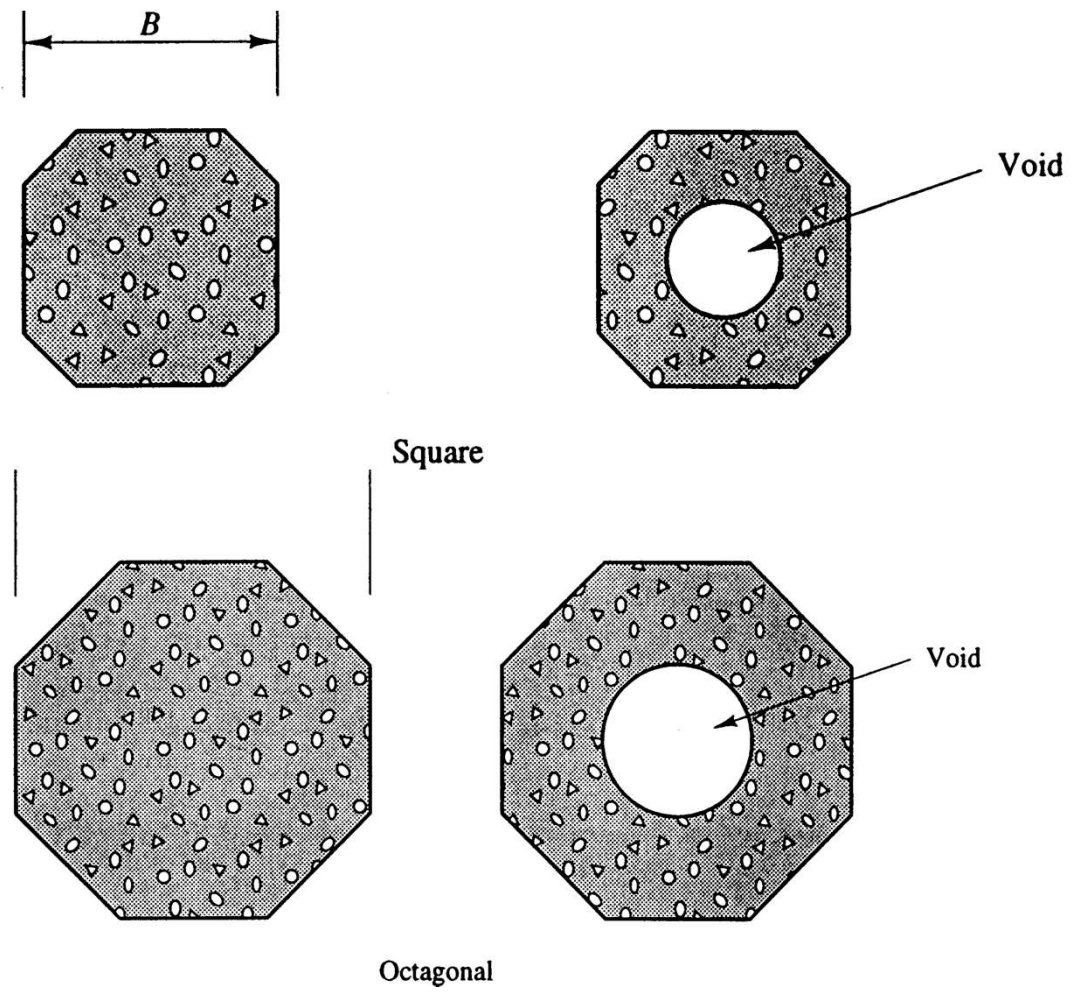
- 100-150 ft length
- 8-36 in diameter
- closed or open end



# Concrete Piles

- Reinforced Concrete
- Pre-stressed concrete
- Do not tolerate hard driving conditions
- Square or Octagonal Section
- 40-400ft long
- 10-24 inches width
- 100-800 kip working loads

# Concrete Piles

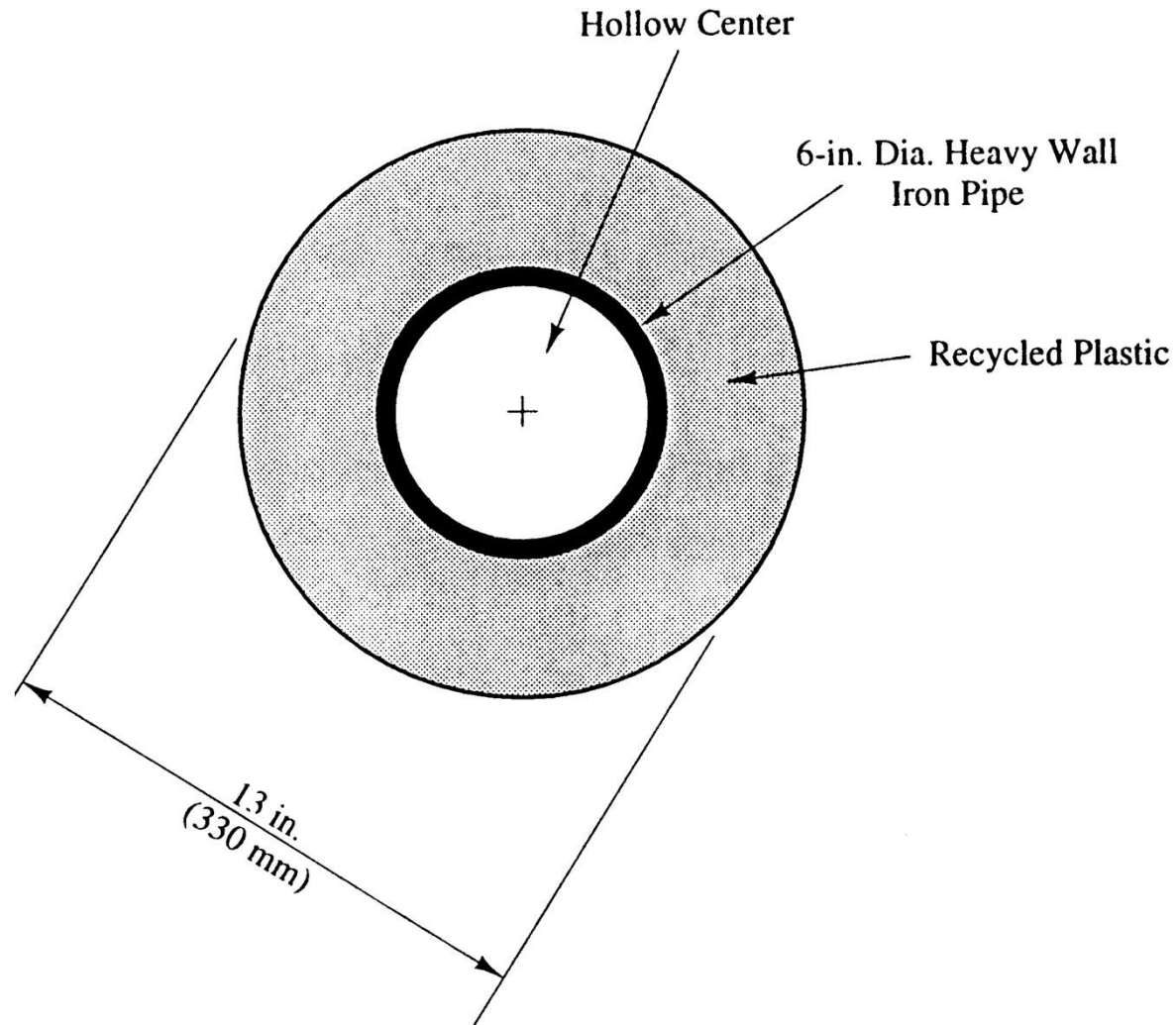




# Composite Piles

- Concrete filled Steel Pipe Piles
  - Greater uplift capacity because of increased weight
  - Increased shear and moment capacity
- Plastic-Steel Composite Piles
  - Used in marine environments to increase resistance to borers, decay, abrasion
  - stronger than timber

# Composite Piles



# Construction Methods and Equipment

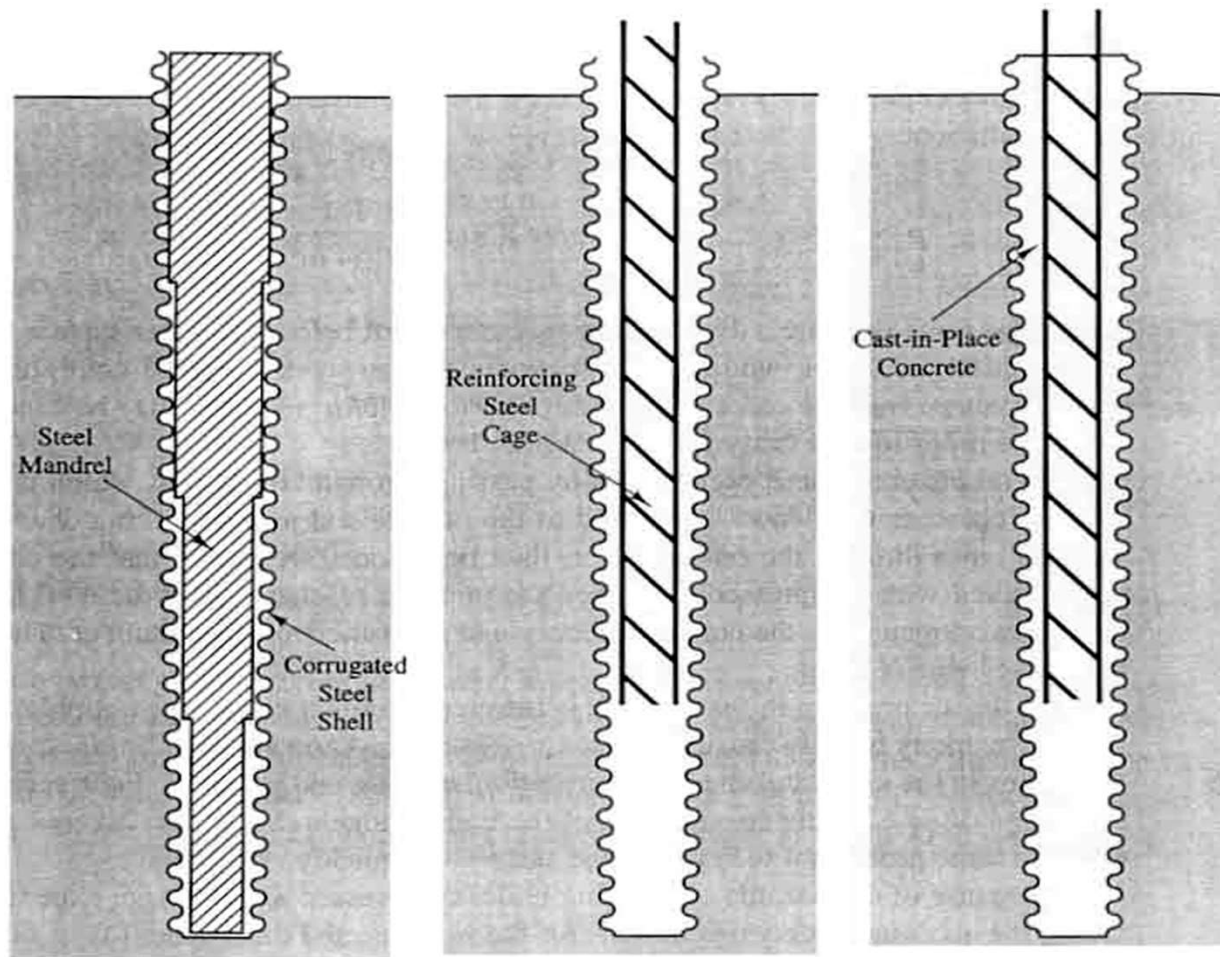
- Pile driving rigs
  - raise and support pile
  - has “leads” to guide the pile
  - hydraulic or cable actuators move “leads” into to desired alignment
  - supports the hammer

# Pile Driving Hammers

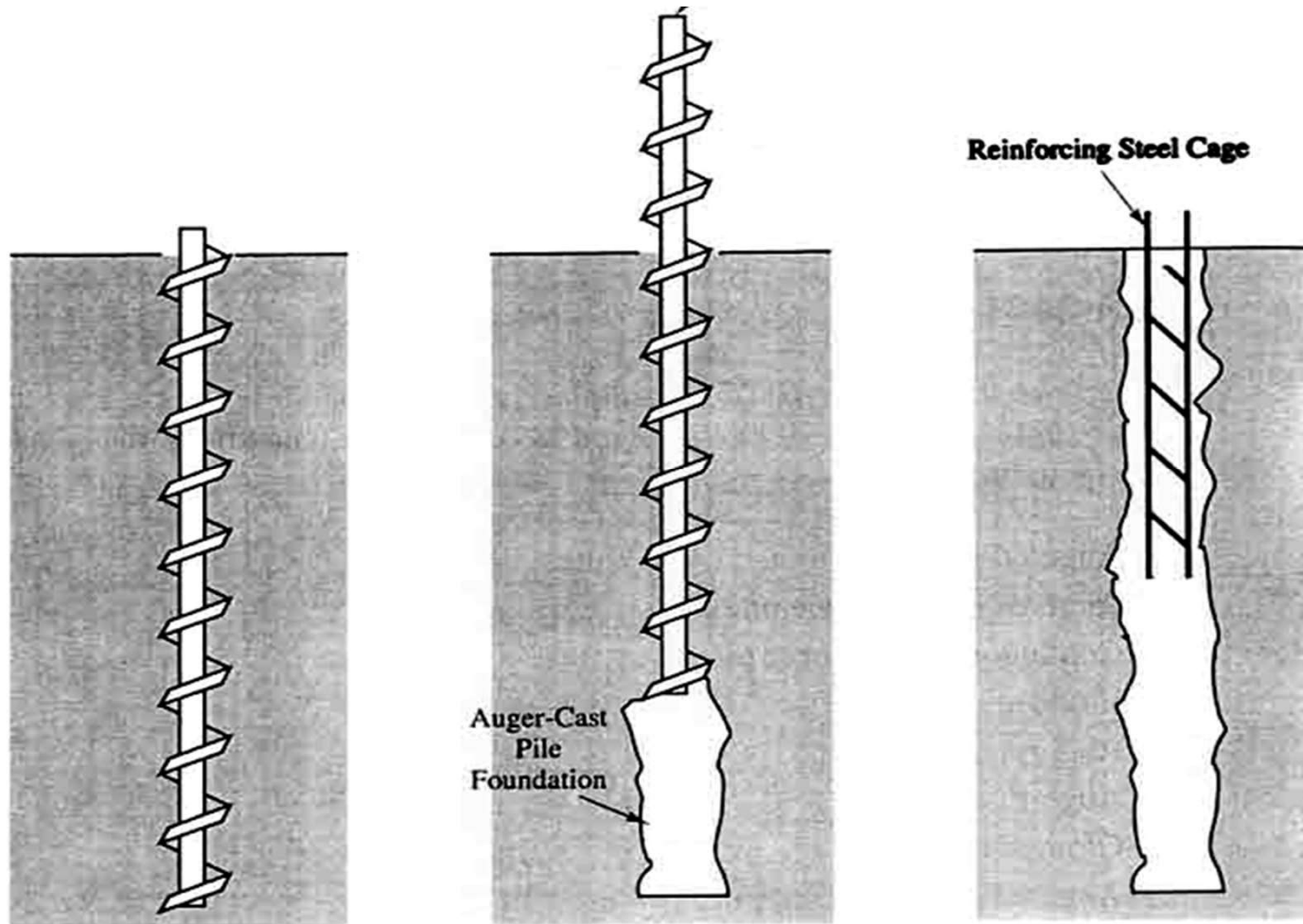
- Drop Hammers
  - rarely used in US now
  - 3-12 blows/min
- Steam, Pneumatic Hydraulic Hammers
  - 60 blows/min
- Diesel Hammers (40-55 blows/min)
- Vibratory Hammers (150 Hz)



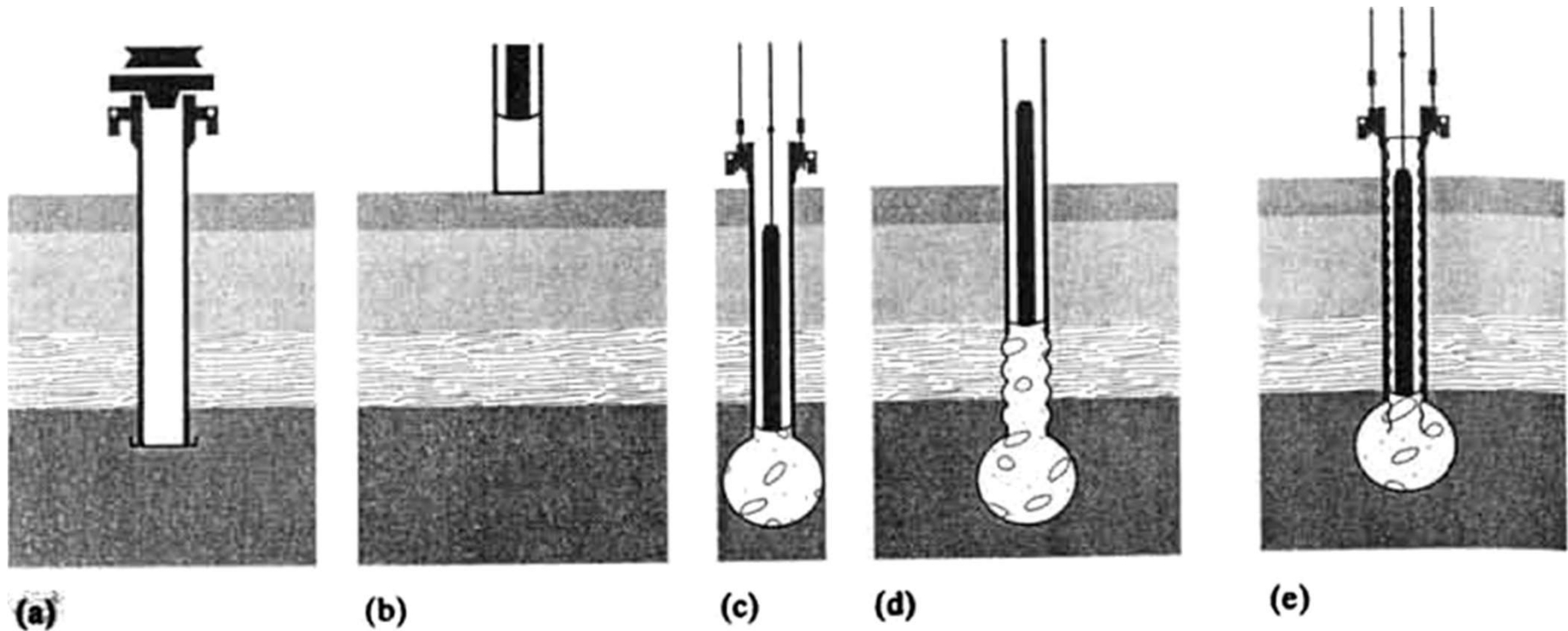
# Thin Shells Filled with Concrete



# Auger-Cast Piles



# Pressure Injected Footings



# Type of Piles According to Construction Methods

- Driven – Prefabricated Members driven into ground
- Bored– Drill Cylindrical Hole and insert reinforcing & fill with concrete
- Caissons – Prefabricated box or cylinder sunk into ground and filled with concrete

# Type of Piles According to Construction Methods (Cont.)

- Mandrel driven shells – Thin corrugated steel shells driven into ground and filled with concrete
- Auger Cast Piles – Drill a slender cylindrical hole with hollow-stem auger and then pump grout through auger hole while auger is slowly retracted
- Pressure Injected Footings – Cast in place concrete that is rammed into the soil using a drop hammer

# Driven Piles versus Bored Piles

## Driven Piles

Precast Members driven into soil

Timber, steel, prestressed concrete, composite

Maximum diameter~16 in.

Used in groups “pile groups”

## Bored Piles

Cast-in-situ

Reinforced concrete

Can be as large as 6-8 ft diameter

Used singly

# Driven Piles versus Bored Piles

## Driven Piles

- Improve soil properties
- Not Effected by water
- It is good if there are part of piles above ground level

## Bored Piles

- use in hard and dense soil















أهداء من  
م/ احمد صابر  
أحد خريجي الكلية



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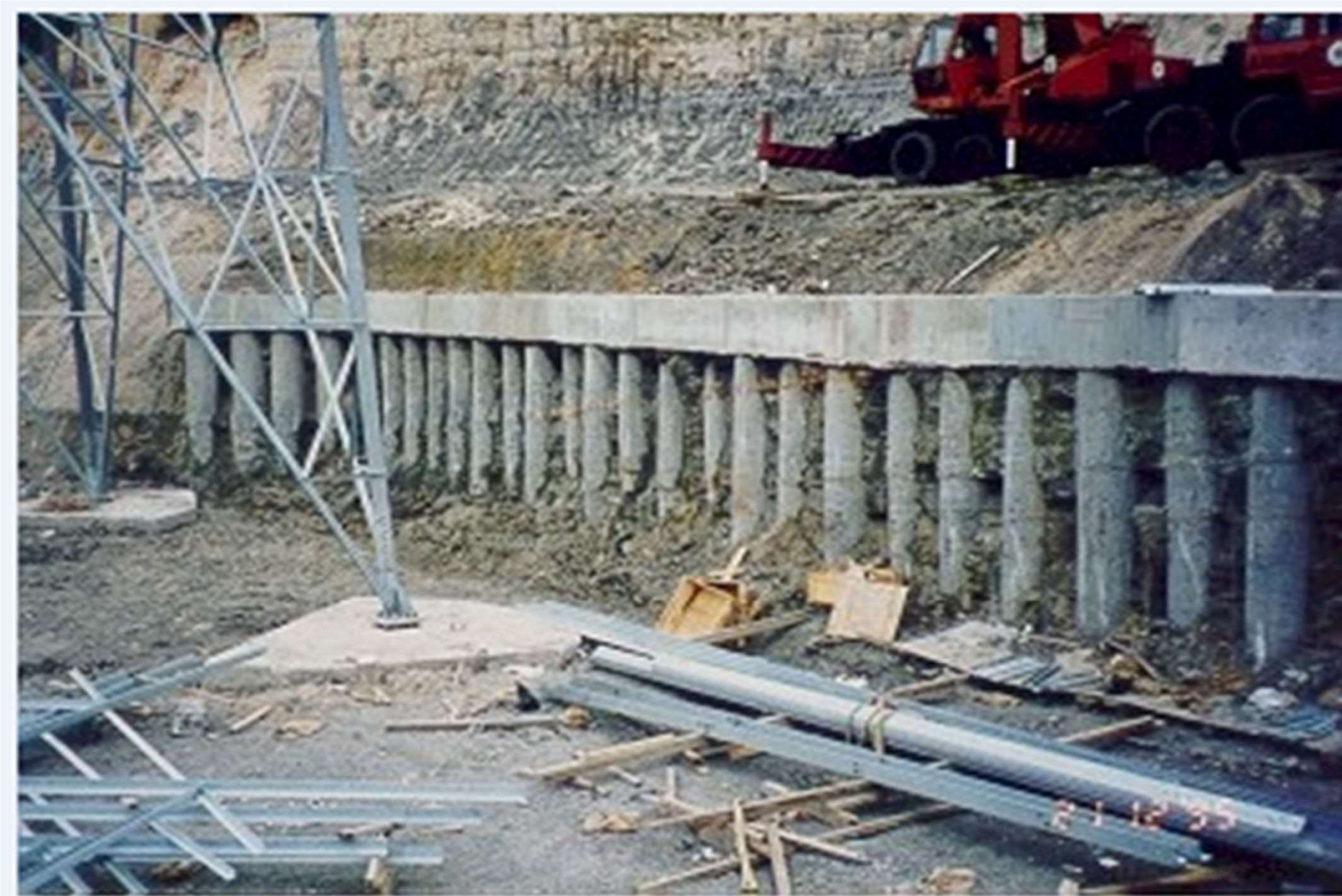


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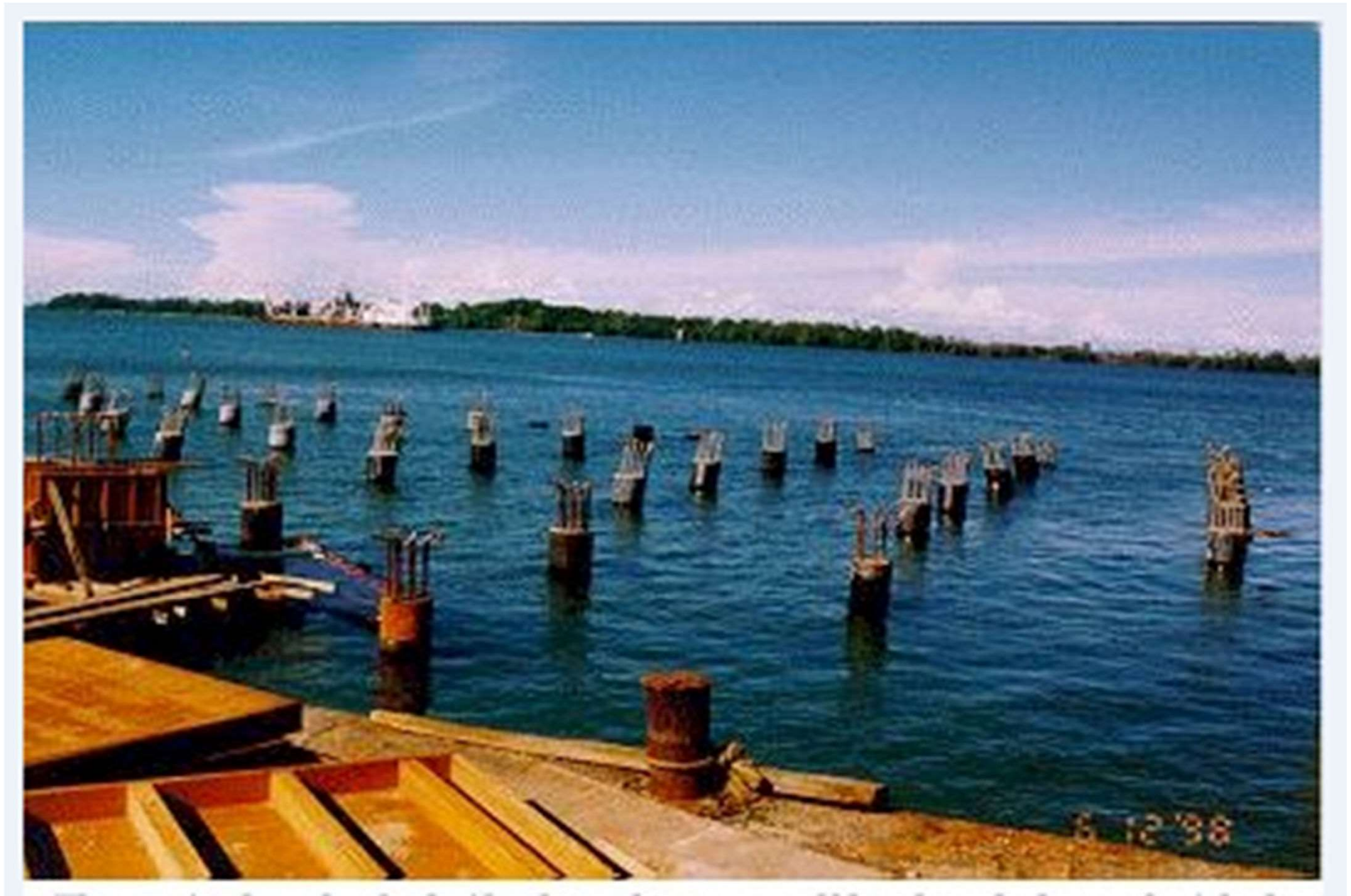












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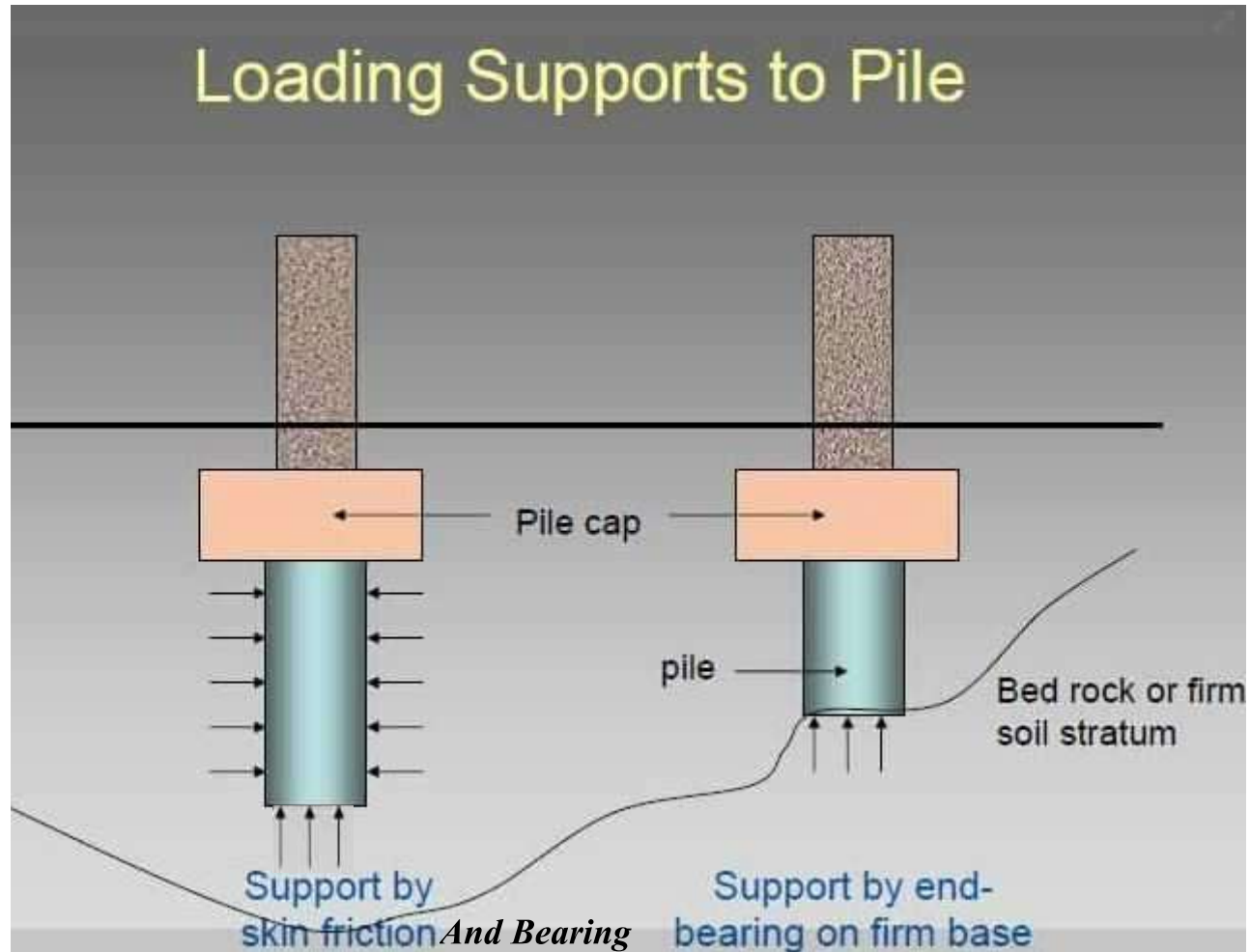


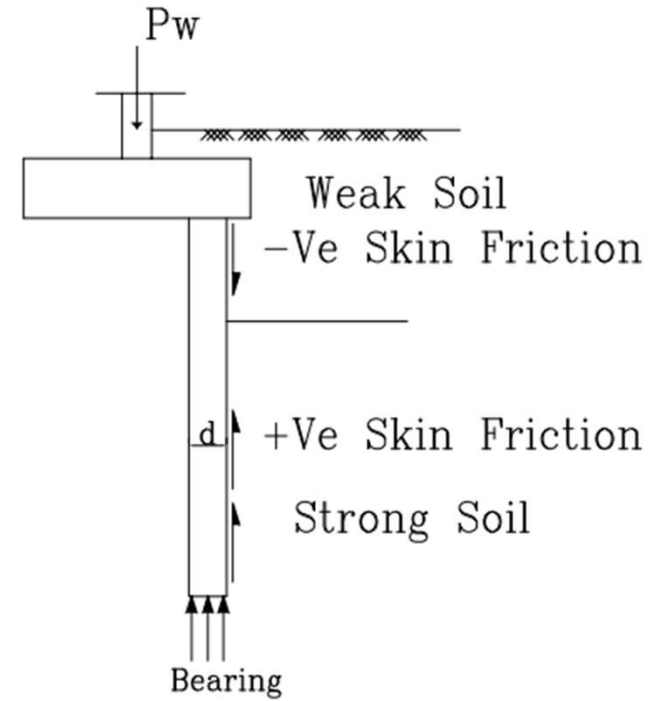
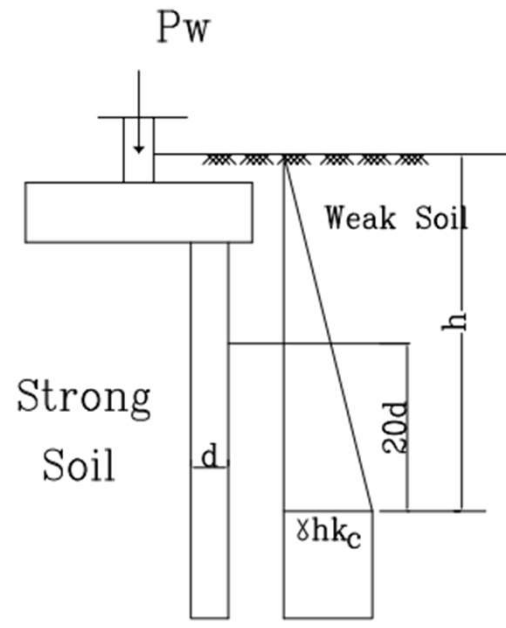
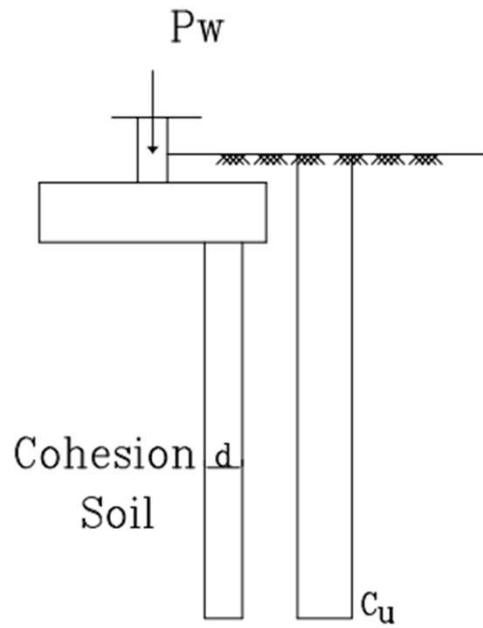






## Loading Supports to Pile





$$P_{all/pile} = \left( \frac{p_b * A_b + p_{s(+ve)} * A_s}{F.S} \right) \eta - p_{s(-ve)} * A_s \quad t \quad (1)$$

$A_b$  Base Area =  $\pi d^2 / 4$  For Circular pile

$A_s$  Surface Area =  $\pi d l$  For Circular pile

$\eta$  Factor=1 if pile supported on sand

Factor if pile supported on clay depends on pile length  
pile distance, and number of piles see fig (4-22)

F.S = 3 IF Loads (D+L)

F.S.= 2.50 IF Loads (D+L+W)

F.S.= 2.00 IF Loads (D+L+E)

a- If pile supported on sand

$$p_b = p_0 * (N_q - 1) \quad t/m^2 \quad ( \text{Bearing stresses} )$$

$$p_0 = \sum \gamma h$$

العلاقة بين معامل قدرة التحميل  $N_q$  وزاوية الاحتكاك الداخلي  $\phi^\circ$

لتربة غير متماسكة الحبيبات

$\phi^\circ$ (degree)	25	30	35	40
$N_q$	15	30	75	150

$$\phi^\circ (\text{Driven pile}) = \frac{\phi^\circ \text{ measure} + 40}{2}$$

$$\phi^\circ (\text{Bored pile}) = \phi^\circ \text{ measure} - 3$$



$$p_s = \sum \gamma h K_C \tan \delta \quad \underline{t/m^2} \quad (\text{Skin Friction stresses})$$

$$K_C = 0.70 \text{ up to } 1.50 \text{ for Driven piles} \quad \text{Take } K=1$$

$$K_C = 0.70 \text{ up to } 1.00 \text{ for Bored piles} \quad \text{Take } K=0.70$$

$$\delta = \emptyset^\circ \quad \text{For Concrete roughness surface}$$

$$\delta = \frac{3}{4} \emptyset^\circ \quad \text{For Concrete piles}$$

$$\delta = 20^\circ \quad \text{For Steel piles}$$

b- If pile supported on Clay

$$p_b = N_C * C_u \quad \underline{t/m^2} \quad (\text{ Bearing stresses } )$$

$$C_u \quad \underline{t/m^2} \quad \text{Unconfined cohesion}$$

$$N_C = \underline{5(1+0.20 \frac{B}{L}) (1+0.20 \frac{D}{B})} \not\geq 9 \quad \text{B,L pile dimensions= Pile Diameter for circale}$$

$$D/B \not\geq 2.50$$

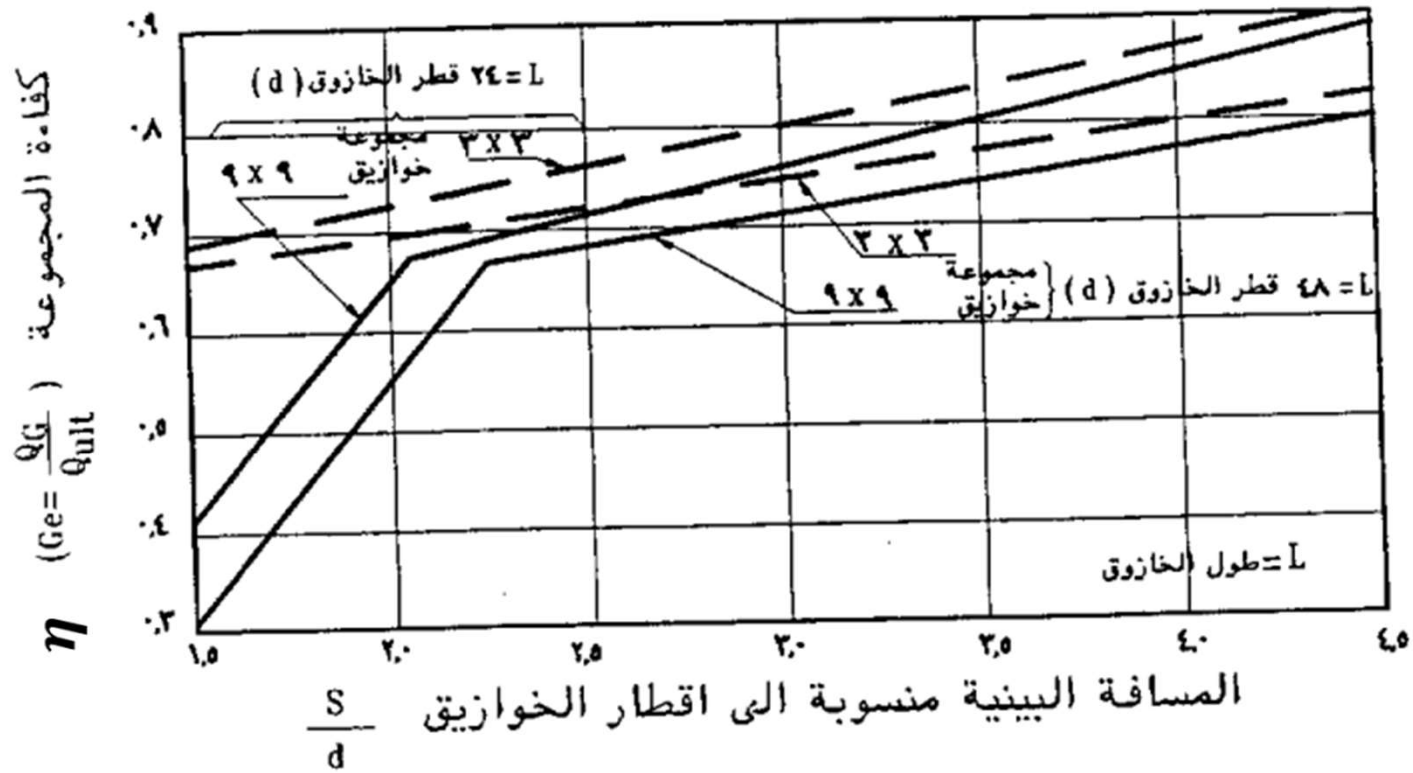
D= Distance from ground to pile end(foundation depth)

$$p_s = \alpha * C_u \quad t/m^2 \quad \not\geq \mathbf{10t/m^2} \quad (\text{ Skin Friction stresses } )$$

$\alpha$  Factor as Shown

$\alpha$	Driven	Bored
<u>V.Soft Clay &amp; Soft</u>	1	1
Medium	0.75 → 1.00	0.40
Stiff	0.475 → 0.75	0.35
<u>V.Stiff</u>	0.325 → 0.475	0.30

take small value for safty



شكل رقم (٤-٢٢): كفاءة مجموعات الخوازيق في التربة الطينية

$$G_e = 1 - \theta \frac{(n - 1) m + (m - 1) n}{90 m n}$$

Where:

$G_e$  = Group efficiency;

$\theta$  =  $\tan^{-1} (D/S)$  in degrees;

$D$  = Pile diameter (m);

$S$  = Pile spacing (m);

$n$  = Number of piles in a row;

$m$  = Number of pile rows

$$P_{conc/pile} = 0.20F_c A_b * Factor - (-ve Skin Friction)A_s \quad t \quad (2)$$

Factor = 1.00                      If pile not Reinforced.

Factor = 1.15                      If pile Reinforced.

$P_{all/pile}$  Take Smaller from 1, 2

$$\text{No. Of Piles} = \frac{\text{Column Loads (DL + LL)} * 1.05}{p/\text{pile}}$$

### Check

P<sub>actual</sub> / pile =

$$= \frac{\text{Column Loads} + \text{O. W Cap} + \text{O. W. Soil above cap} + (-\text{Ve Skin Friction on Cap}) + \text{Surcharge}}{\text{No Of Piles}}$$

- If Piles Support on Sand Called Bearing Piles
- If Piles Support on Clay Called Friction Piles

## 1- End bearing piles: خوازيق ارتكاز

**Tip point carries most of the load.**

نقطة الارتكاز تنقل معظم حمل الخازوق.

## 2- Friction Piles: خوازيق احتكاك

**Side friction carries most of the load.**

معظم حمل الخازوق ينتقل للتربة عن طريق الاحتكاك السطحي



جدول رقم (٣-٨) قيم زاوية مقاومة القص للتربة غير المتماسكة

زاوية مقاومة القص	الكثافة النسبية Relative density		اختبار الاختراق القياسي
$\phi^\circ$	$D_r = \frac{e_{max} - e}{e_{max} - e_{min}}$	الوصف	عدد الدقات N / ٣٠ سم
٢٧ - ٣٠	٠,١٥ - صفر	سائبة جداً very loose	صفر - ٤
٣٠ - ٣٢	٠,٣٥ - ٠,١٥	سائبة loose	٤ - ١٠
٣٢ - ٣٦	٠,٦٥ - ٠,٣٥	متوسطة الكثافة medium	١٠ - ٣٠
٣٦ - ٤٠	٠,٨٥ - ٠,٦٥	كثيفة dense	٣٠ - ٥٠
٤٠ <	٠,٨٥ <	كثيفة جداً very dense	٥٠ <

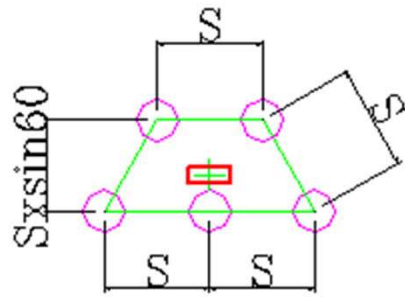
حيث:

N عدد الدقات من اختبار الإختراق القياسي

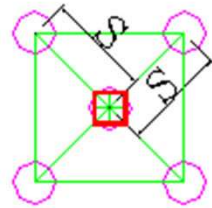
جدول رقم (٣ - ٧) القيم الاسترشادية لمقاومة التماسك غير المصروفة  
للتربة الطينية ( $c_u$ ) \*

مقاومة التماسك $c_u^*$		القولم		الختبار الاختراق القياسي
كجم / مم <sup>٢</sup>	ك.نيوتن / م <sup>٢</sup>	$I_c = \frac{w_L - w}{w_L - w_P}$	الوصف	عدد الدقات (N) / ٣٠ مم
صفر - ٠,١٢٥	صفر - ١٢,٥	صفر - ٠,٥	ضعيف التماسك جدا very Soft	صفر - ٢
٠,١٢٥ - ٠,٢٥	١٢,٥ - ٢٥	٠,٦٢٥ - ٠,٥	ضعيف التماسك Soft	٢ - ٤
٠,٢٥ - ٠,٥	٢٥ - ٥٠	٠,٧٥ - ٠,٦٢٥	متوسط التماسك medium	٤ - ٨
٠,٥ - ١	٥٠ - ١٠٠	١,٠٠ - ٠,٧٥٠	متماسك Stiff	٨ - ١٥
١ - ٢	١٠٠ - ٢٠٠	( $w_s = w$ ) - ١,٠٠	تجيد التماسك جدا very Stiff	١٥ - ٣٠
٢ <	٢٠٠ <	( $w_s > w$ )	صلد hard	٣٠ <

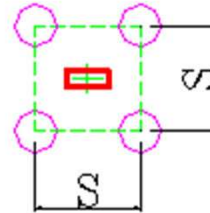
$c_u^* = \frac{1}{2}$  مقاومة الضغط الغير محاط ( $q_u$ ) unconfined compressive strength



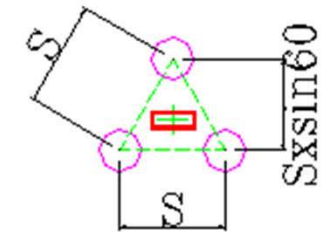
5 Piles



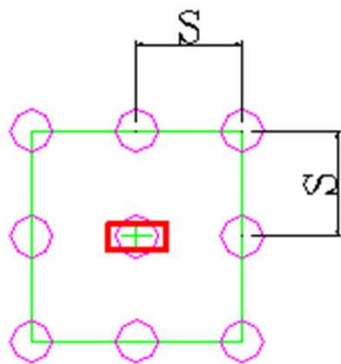
5 Piles



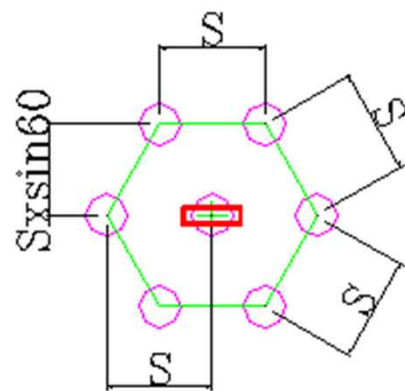
4 Piles



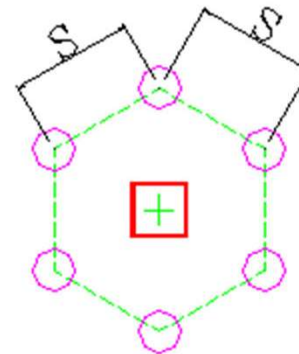
3 Piles



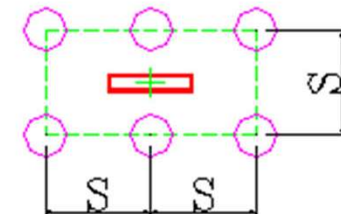
9 Piles



7 Piles



6 Piles



6 Piles

+ CG Of Col & Pile Group

If Piles Rested on Clay or there are clay on effective zone ,

Check Block Failure

$$F.S = \left( \frac{p_b * A_b + p_{s(+ve)} * A_s - p_{s(-ve)} * A_s}{P_n} \right) \geq 2$$

$A_b$	Base Area	$A*B$
$A_s$	Surface Area	$2*(A+B)*L$



